# Experimental investigations of concrete on slag Portland cement as a coating material for agricultural aerodromes

Oleksii YANIN<sup>1, a</sup>, Tetiana YEMELIANOVA<sup>1, b</sup>, Svetlana NOVIKOVA<sup>1, c</sup>

<sup>1</sup>**06** Stritenskaya, 23, Kherson, 73006, Ukraine <sup>a</sup> yanin\_a@ukr.net, <sup>b</sup> e.tatyana.2014@ukr.net, <sup>c</sup> novikova\_svetla@ukr.net

Keywords: concrete, sulfate-resistant slag Portland cement, hardening conditions, water-cement ratio, lignosulfonates, surface-active additives, strength, frost resistance, agricultural roads, aerodromes.

**Abstract.** The results of concrete testing on sulphate-resistant slag Portland cement for rigid coating of agricultural roads and aerodromes are presented in the paper. The results of experimental investigations of samples in the form of cubes on compressive strength and samples in the form of prisms on bending strength under different hardening conditions and at different water-cement ratios are presented. Selection of hardening conditions of concrete on sulphate-resistant slag Portland cement was made based on achievement of quality indicators no worse than in concrete on Portland cement. The expediency of introducing into the concrete organic surface-active additives - lignosulfonates in order to achieve the required strength is grounded. It is proved experimentally that the proposed concrete has sufficient frost resistance.

# Introduction

The effectiveness of the use of rigid coatings made from prefabricated concrete products on agricultural aerodromes is not fully investigated. The experience of their practical using at aerodromes deserves further study. In addition, existing prefabricated structures used for the construction of aerodrome and road surface coatings are ineffective from the economic point of view for agricultural aviation aerodromes in most cases. Therefore, the using of new building materials and the development of lightweight economic coatings made of prefabricated concrete elements for using at agricultural aerodromes require serious consideration.

## **Formulation of the Problem**

A promising direction in the construction of agricultural aerodromes coatings is the using of concrete on slag Portland cement binder [1], which favorably differs from Portland cement with greater environmental friendliness, affordability, reduced energy consumption and cost.

Sufficient number of works has been devoted to solving the problem of improving the performance of concrete [2-14]. The using of Portland cement, modified by complex additives of plasticizing and air-pulling action, contributed to the creation of polyfunctional concrete [3,4]. The introduction to Portland cement of pozzolana, carbonate additives and blast furnace slag contributes to increasing its activity by optimizing the particle size distribution [5-7]. Studies [8] have shown that when composite binder is used, it is possible to obtain high strength concrete at early hardening times. At the same time, the use of slag opens the possibility to obtain concrete, which is not inferior to concrete on conventional Portland cement [9].

Therefore, the determination of frost resistance and strength characteristics of concrete on slag Portland cement binder during compression and bending, and justification of the possibility, effectiveness and expediency of its use for rigid agricultural roads and aerodromes, is actual task.

The aim of the work is to determine the strength characteristics of concrete based on sulphateresistant slag Portland cement during compression and bending under different conditions of hardening; substantiation of expediency of introduction into the concrete of organic surfactants - lignosulfonates in order to achieve the required strength; experimental investigations of concrete samples on frost resistance.

### **Materials and Methods**

For the experiments, concrete samples were made under different hardening conditions [15]. Sulphate-resistant slag Portland cement of brand 400 was used as a binder for these samples. It has the following physical and mechanical characteristics:

- Water-cement ratio 0.38;

- Flow of the cone 115 mm;

- Fineness of grinding: 91.8% of cement by weight passed through sieve 008;

- Normal density of cement dough 26.50%;

- Timing of setting: the beginning of setting comes in 4 hours 10 minutes after mixing the cement dough, and the end of setting - in 5 hours 30 minutes;

- Compressive strength after steaming 271 kgf /  $cm^2 = 26,6$  MPa;

- Bending strength after steaming 50.7 kgf /  $cm^2 = 4.97$  MPa;

- Content of granular metric blast furnace slag in cement is 48% by weight.

Concrete samples after molding were in the following different hardening conditions:

H -normal hardening at relative humidity of air 90-95% in a special chamber;

C - dry hardening at a relative humidity of air 40-70% in room conditions;

M - normal hardening after thermic humidity treatment in the regime 2 hours + 3 hours + 6 hours at the temperature T = 70  $^{\circ}$  C, where

2 hours - holding the samples before steaming,

3 hours - raising the temperature in the steaming chamber (where the samples are placed) to a given value T,

6 hours - holding the samples in the steaming chamber at a given temperature T;

MC - dry hardening after thermic humidity treatment under the same regime during 2 hours + 3 hours + 6 hours at the temperature  $T = 70 \circ C$ ;

M1 - normal hardening after thermic humidity treatment under the same regime 2 hours + 3 hours + 6 hours at the temperature  $T = 90 \circ C$ ;

MIC - dry hardening on regime 2 hours + 3 hours + 6 hours at  $T = 90 \circ C$ ;

W1 - normal hardening after thermic humidity treatment on regime 1 hour + 2 hours 20 minutes + 3 hours 30 minutes at the temperature  $T = 90 \circ C$ , where

1 hour - holding the samples before the steaming,

2 hours 20 minutes - raising the temperature in the steaming chamber (where the samples are placed) to a given value T,

3 hours 30 minutes - holding the samples in the steaming chamber at a predetermined temperature T;

W1C - dry hardening after thermic humidity treatment on regime 1 hour + 2 hours 20 minutes + 3 hours 30 minutes at the temperature  $T = 90 \circ C$ .

The steaming chamber was switched off after holding the samples. The samples were gradually cooled from temperature T to room temperature.

## **Research Results**

Samples for the experimental determination of the strength limit of concrete were produced of cubic-shaped with size  $10 \times 10 \times 10$  cm. They were tested at the age of 1 day, 3 days, 7 days, 28 days, and 180 days. Samples were mounted on the press and loaded gradually by the force P (Fig.1) [16].



Fig. 1. Testing of concrete on compressive strength

The fracture load for sample corresponds to the limit compressive strength of concrete. Samples for the experimental determination of the tensile strength of concrete at bending were made in the form of a prism with size 10 x 10 x 40 cm. They were tested at the age of 1 day, 7 days, and 28 days. The test scheme can be represented as follows: (Fig. 2) [16]



Fig. 2. Testing of concrete on strength at bending: 1 - beam for transferring of a load to the sample; 2 - sample

The samples were loaded gradually by a load P. The fracture load for the prism  $P = P_{fr}$ , corresponds to the limit strength of concrete at bending  $f_{ctd}$ . The dependence of  $f_{ctd}$  on  $P_{fr}$  can be represented as follows:

$$f_{ctd} = \delta \frac{P_{fr} \cdot L}{b \cdot h^2},\tag{1}$$

where L, b, h – the dimensions of the sample shown in Fig. 2,  $\delta$  – the calculated coefficient.

The samples were tested by groups, and CK brand were assigned to them which the test number was added to. The results of testing of concrete for strength are presented in table.1, 2.

Analyzing the test results of samples of 1-3 groups (Table 1), we can conclude that their bending strength on the 28th day was insufficient. This caused the need for testing with low water-cement ratio (4-6 sample groups). The possibility of reducing the water-cement ratio was ensured by the introduction into the concrete mixture of organic surface-active additives. Lignosulfonates (LSTs) were selected from them, as the most common and available industry wastes [17, 18]. It was found that the lowering of water-cement ratio had a positive effect not only on the compression strength of

concrete, but, very importantly, on the bending strength of concrete (Table 2). Besides, the ratio of compressive strength to bending strength in the samples made with the addition of lignosulfonate is less than in samples made without the additive, which is also a positive factor.

Table 1. The results of testing of concrete samples of 1-3 groups	, made on the basis of slag Portland
cement of brand 400, on compressive	strength

	Composition of 1m <sup>3</sup> concrete mixture						Limit of compressive strength, MPa, for						
Brand of samples	Cement (C), kg	Water (W), kg	Sand (S), kg	Crushed stone, kg	LST in % of cement	Hardening conditions	1 day	3 days	samples a samples a solution of the samples a samples a sample sam	28 days	180 days		
The first group of samples													
CK1						Н	-	9,2 9,6	17,2 17,2	25,4 27,9	32,4 32,8		
CK2				1050	_	С	-	9,6 10,0	16,8 17,6	22,5 22,9	-		
CK5	450	195	600			М	25,8 26,3	_	34,4 35,2 35,6	44,6 46,3 46,3	49,6 49,6 50.4		
CK6						МС	_	_	37,2 37,6	45,4 45,4 47,1	47,1 47,5 47,5		
		•		The	second gro	oup of s	amples						
CK1a						Н	-	12,0 12,8	18,4 18,8	27,9 28,3	-		
СК3	450	190	600	1050	-	M1	27,1 27,1	-	30,0 33,1	38,0 38,4 39,2	40,4 40,0		
CK4						M1C	-	-	34,0 34,4 34,8	40,8 41,3 41,7	46,3 46,7 46,7		
				Th	e third grou	up of sa	mples						
CK1b						Н	-	10,0 10,4	19,6 20,0	30,0 32,0	-		
СК9	450	190	600	1050	-	W1	25,4 25,8 29,2	-	32,4 32,8 32,8	36,0 36,4 38,0	43,3 47,5 47,9		
CK10						W1C	-	-	34,0 33,6	39,6 41,3 43,3	44,4 44,2 44,6		

Influence of the hardening conditions of concrete on its strength characteristics is investigated. For this purpose, the results of testing the samples of brand CK5 without additives from the first group were compared with the results of testing the samples of brand CK3 with the addition of LST from the second group. The hardening conditions of the samples of brand CK3 (M1) with the additive were more rigid than the hardening conditions of the samples without additive CK5 (M). Comparing the experimental strength characteristics of these samples, it can be noted that the softer

regime provided higher compressive strength to the 28th day, and their bending strength was almost the same. The samples of brand CK9 from the third group were in even more rigid hardening conditions (W1) in comparison with the samples of brand CK5. As expected, their compressive strength was lower than that of the CK5 samples. In this case, the bending strength of the samples of brand CK9 was increased.

Table 2.	The res	ults of	ftesting	of concrete	samples,	made on	the	basis	of slag	Portland	cement	t of
				brand 400	), on band	ing streng	gth					

group amples	oles	Com	positio	n of 1m	<sup>3</sup> concrete	mixture		ment	sition,	Limit of bending strength, MPa, for			
	amp	(),	Ċ.	g	Crushed stone, kg LST in % of cement tement mardening conditions Water-cemer	of	uing ons			samples aged			
Sample g	Brand of s	Cement (C kg	Water (W kg	Sand (S), I		Water-ce ratic	Cone depc cm	1 day	7 days	28 days			
1	CK5	450	195	600	1050	-	М	0,43	3	2,1 2,2	-	2,9 3,1	
2	CK3	450	190	600	1050	-	M1	0,42	2,5	1,9 2,1	-	3,1 2,9	
3	CK9	450	190	600	1050	-	W1	0,42	3	2,1 2,1	2,8 2,8	3,3 3,9	
4	CK11	450	170	600	1050	0,2	М	0,38	4,2	3,4 3,6	-	4,5 5,1	
5	CK13	450	170	600	1050	0,2	M1	0,38	3,5	2,9 3,2	-	4,5 4,65	
9	CK27	300	180	700	1100	-	W1	0,6	2,5	1,8 1,8	-	2,5 2,5	
10	CK29	300	180	700	1100	-	M1	0,6	2,5	-	-	2,5 2,7	

The check of the marked empirical dependences of strength concrete on the hardening conditions when introduced into the concrete of surface-active additives was made. For this purpose, the testing results of the brand samples CK11 from the fourth group were compared with the testing results of brand samples CK13 from the fifth group. The hardening conditions of the brand samples CK13 (Ml) were more rigid than the hardening conditions of the samples CK11 (M). Analyzing the test results, we can conclude that a softer regime provided higher compression strength to the 28th day for the samples with additives, as well as for the samples without additives. The bending strength of the CK13 samples was lower than that of the CK11 samples.

The check of the marked empirical dependences for other ratio between cement and filler was made. For this purpose, 7-10 groups of samples were made. In that the amount of cement per 1  $m^3$  of concrete was reduced by 1.5 times, and the amount of water, sand and crushed stone remained practically unchanged in comparison with 1-6 groups of samples. As expected, the absolute values of the compression and bending strengths in the samples of 7-10 groups were smaller than in the samples of 1-6 groups.

Influence of hardening conditions of concrete on its strength characteristics was also considered. For this purpose, we compared the testing results of the brand samples CK27 from the ninth group with the testing results of the brand samples of CK29 from the tenth group. The hardening

conditions of the CK27 (W1) samples were more rigid than the hardening conditions of the CK29 (M1) samples. Comparing the experimental strength characteristics of these samples, we can conclude that they are almost identical with the barely noticeable tendency that a softer hardening mode is the most acceptable.

Analyzing the testing data as a whole, it can be noted that concrete, made on the basis of slag Portland cement, when included in it surface-active additives (4-6 groups of samples) has no worse strength characteristics than concrete made on the basis of conventional Portland cement, and it can be used for the construction of prefabricated coatings for agricultural aerodromes.

The samples having the shape of a cube, with size 10 x 10 x 10 cm, for the study of concrete on slag Portland cement binder for frost resistance were made. They were subjected to thermic humidity treatment under different regimes and then they were hardened under normal conditions. After a set of strengths for 28 days, they were placed in a freezer where they were subjected to freezing and thawing cycles. One cycle corresponds to 4 hours of freezing at a temperature of T = -20-25 ° C and 4 hours of thawing, after which the samples were filled with water at room temperature. The water was drained after 4 hours and the cycle repeated again [16].

Frost resistance of concrete was visually assessed by a ten-point scale proposed by the professor S.V. Shestoperov. As is known from the large number of experiments carried out on concrete samples from different cements, concrete retains its bearing capacity (does not suffer destruction), if it has scored at least 6 points on this scale [19].

The testing results of concrete samples for frost resistance, which made on the basis of sulfate-resistant slag Portland cement, are given in table 3.

Brand of concrete samples	Content in 1m <sup>3</sup> of concrete mixture			ment	ing ons	r of es	Number of points (b) in non-fracture sample cubes (c) after finishing of the next number of freeze-thaw cycles					
	Cement (C), kg	Water (W), kg	LST in % of cement	Water-ce ratic	Harden conditi	Numbe sampl	65	220	430	630		
СК3	450	190	-	0,42	M1	6	10b-6c	9-10b-6c	9b-5c 5b-1c	9b-2c 7b-1c		
CK5	450	195	-	0,43	М	6	10b-6c	9-10b-6c	9b-2c 5-6b-4c	7-8b-1c		
СК9	450	190	-	0,42	W1	6	10b-6c	9-10b-6c	9b-3c 8b-2c 5b-1c	8b-1c		
СК13	450	170	0,2	0,38	M1	6	10b-6c	9-10b-6c	8-9b-6c	8b-1c 6b-5c		
CK11	450	170	0,2	0,3U	М	6	10b-6c	9b-6c	9b-6c	9b-5c 7b-1c		
CK17	450	175	0,2	0,39	W1	6	10b-6c	9-10b-6c	9-10b-6c	9b-4c 8b-2c		
CK21	300	190	-	0,63	M1	6	10b-5c 7b-1c	8-9b-1c 7b-1c 6b-1c	4b-1c	-		
СК23	300	195	-	0,65	W1	6	10b-6c	8-9b-3c 7b-1c	4-5b-3c	-		

Table 3. The results of testing of concrete samples, made on the basis of slag Portland cement of<br/>brand 400, on frost resistance

As one can see from table 3, concrete samples of brands CK3, CK5, CK9, CK11, CK13, CK17 have high frost resistance (from 6 to 9 points) even after 630 cycles of freezing and thawing.

The samples of grades CK21 and CK23 with a cement content of 300 kg per 1  $m^3$  of concrete mixture is characterized by slightly lower frost resistance. However, they can withstand at least 200 cycles of freezing and thawing. It can be concluded that the frost resistance of concrete on slag Portland cement is quite sufficient for the construction of prefabricated coatings of agricultural aerodromes.

#### Conclusions

Experimental studies of concrete on sulphate-resistant slag Portland cement of brand 400 have shown that the use of this material for the construction of prefabricated coatings for agricultural aerodromes allows to save Portland cement and to use of secondary raw materials.

Such concretes can provide a class of strength on compression C25 / 30 and of strength on bending C<sub>t</sub>4.0 at the consumption of slag Portland cement 450 kg per 1 m<sup>3</sup> of concrete mixture.

However, reliable data on the compressive strength and the bending strength are obtained in the case of introduction into the concrete mixture of non-deficient plasticizing additives - lignosulfonates in the amount 0.2% from weight of cement. The bending strength is insufficient if plasticizing additives are absent.

The concrete on sulphate-resistant slag Portland cement has a high frost resistance. Concrete of optimal composition can withstand over 600 cycles of freezing and thawing.

#### References

- [1] Volkov M.I. (1989) Metallurgicheskiye shlaki v dorozhnom stroitel'stve. Moskva: Avtotransizdat, 183 pp.
- [2] Korneeva, I., Neutov, S., Suriyaninov, M. Experimental studies of fiber concrete creep (2017) MATEC Web of Conferences 2017. — Vol. 116, 2017: Structures, Buildings and Facilities — [02021, 6 p.].
- [3] Sanyts'kyy M. A. (2016) Betony polifunktsional'noho pryznachennya na osnovi kompozytsiynykh tseolitvmisnykh portlandtsementiv / M. A. Sanyts'kyy, T. P. Kropyvnyts'ka, I. M. Hev`yuk, M. V. Kotiv // Visnyk Natsional'noho universytetu "L'vivs'ka politekhnika". Teoriya i praktyka budivnytstva. № 844: 188-193.
- [4] M. A. Sanyts'kyy. (2016) Nanomodyfikovani portlandtsementni kompozytsiyi z vysokoyu mitsnistyu u rann'omu vitsi / M. A. Sanyts'kyy, U.D. Marushchak, T.A. Mazurak // Budivel'ni materialy, vyroby ta sanitarna tekhnika: nauk.-tekhn. zb. Vyp. 57: 147–154. ISSN 2413-7693.
- [5] Markiv T., Huniak O., Sobol Kh. (2014) Optimization of concrete composition with addition of zeolitic tuff. Visnyk NULP. Theory and practice of buildings. № 781: 116–121.
- [6] Geviuk I., Kropyvnytska T., Sanytsky M. (2015) Composite Portland cement with the addition of natural zeolite and limestone. Resource-economical materials, structures and buildings. Rivne. № 31: 149–156.
- [7] The principles of sustainable development strategies in the cement industry (2015) / T. Kruts,
  I. Geviuk, M. Sanytsky, T. Kropyvnytska // Building materials and article. № 3:14–17.
- [8] Barabash I.V., Zubchenko N.A. (2015) Svoystva betonov na aktivirovannom kompozitsionnom vyazhushchem. Vesnik DonbaskoyNatsional'noy akademii stroitel'stva i arkhitektury. Vyp.2015-1(111): 9-13. ISSN 1814-3296.
- [9] Marushchak D. (2015) Shvydkotverdnuchi betony na osnovi portlandtsementiv, modyfikovanykh ul'tradyspersnymy dobavkamy / U.D. Marushchak, B.H. Rusyn, T.A. Mazurak, YU.V. Olevych.// Budivel'ni materialy i vyroby. № 3: 36–39. ISSN 2413-9890.
- [10] Mazurak T.A. (2015) Vplyv dobavok plastyfikuval'no-pryskoryuval'noyi diyi na strukturoutvorennya ta mitsnist' betoniv / Mazurak T.A., Marushchak U.D., Olevych YU.V. ta in. // Visnyk Natsional'noho universytetu "L'vivs'ka politekhnika": Teoriya i praktyka budivnytstva. № 823: 216–222.

- [11] U. Marushchak. (2016) Research of nanomodified Portland cement compositions with high early age strength / U. Marushchak, M. Sanytsky, T. Mazurak, Yu. Olevych //Eastern-European Journal of Enterprise Technologies. № 6/6 (84): 50– 57. ISSN 1729-3774, Scopus.
- [12] U. Marushchak. (2016) Nanomodified Portland cement compositions with alkaline activation
  / U. Marushchak, M. Sanytsky, T. Mazurak, Yu. Olevych // Budownictwo o zoptymalizovanym potenciale energetycznym: Praca zbiorova. № 2(18). P. 61-66. (ISSN 2299-8535, Index Copernicus, Baz Tech).
- [13] Mazurak T.A. (2014) Shvydkotverdnuchi betony na osnovi modyfikovanykh portlandtsementiv / T.A. Mazurak, U.D. Marushchak, I.S. Ivasiv // Naukovyy visnyk NLTU Ukrayiny. Vyp. 24.7: 202–206. ISSN 1994-7836
- [14] Marushchak U.D. Nanomodyfikovani shvydkotverdnuchi portlandtsementy ta betony na yikh osnovi / U.D. Marushchak, YU.V. Olevych, T.A. Mazurak, V.F. Pop //III vseukrayins'ka naukovo-tekhnichna konferentsiya «Suchasni tendentsiyi rozvytku i vyrobnytstva sylikatnykh materialiv». L'viv. 100-102.
- [15] L'vovskiy V. N. (1970) Issledovaniye mekhanicheskikh kharakteristik betona s primeneniyem EVM, statisticheskikh metodov i aktivnykh eksperimentov. Kishinev, 78 pp.
- [16] Ispytaniya dorozhno-stroitel'nykh materialov. Laboratornyy praktikum (1985) / I. M. Glushko,
  V. A. Zolotarev, N. F. Glushchenkov i dr. Moskva: Transport, 200 pp.
- [17] Ratinov V.B., Rozenberg T.N. (1989) Dobavki v beton. Moskva: Stroyizdat, 188 pp.
- [18] Sergeyeva V.N., Tarnarutskiy G.M. i dr. (1979) Lignosul'fonaty kak plastifikatory tsementa // Khimiya drevesiny. № 3: 3-12.
- [19] Shestoperov S.V. (1966) Dolgovechnost' betona transportnykh sooruzheniy / S.V. Shestoperov. Moskva: Transport, 500 pp.